



Structural Addendum #1

1.0 REFERENCE STRUCTURAL DRAWING S1-01 (RE-ISSUED)

- .1 Concrete mix for apparatus bay slab on grade added. See clouded area.
- .2 BE elevations edited in the Bulk Excavation Plan. See clouded areas.
- .3 Underside of footing note edited. See clouded area.

2.0 REFERENCE STRUCTURAL DRAWING S2-01 (RE-ISSUED)

- .1 Steel Column Schedule has been edited. See clouded area.

3.0 REFERENCE STRUCTURAL DRAWING S4-05 (ISSUED)

- .1 Drawing is now included/added to be part of the set of structural drawings.

END OF STRUCTURAL ADDENDUM #1

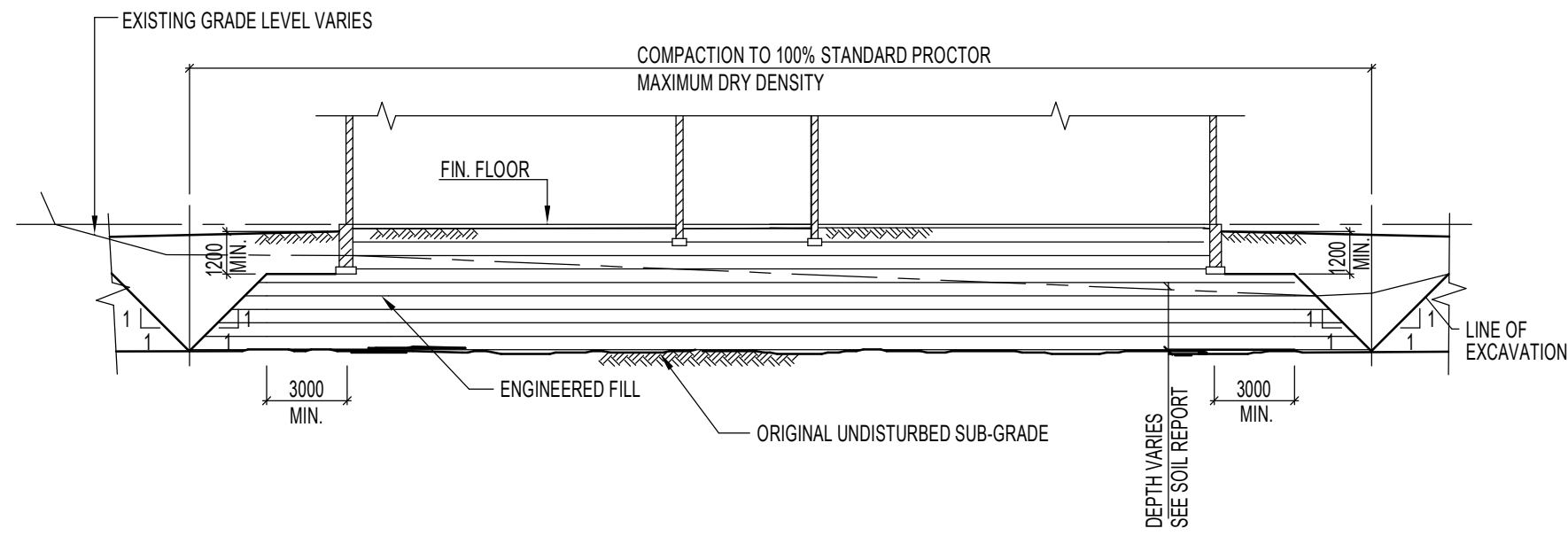
CONCRETE MIX SCHEDULE

EXPOSURE	ELEMENT	MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS (MPa)	EXPOSURE CLASSIFICATION	NOTES
GENERAL NON-EXPOSED CONCRETE (i.e., NOT EXPOSED TO CHLORIDES NOR FREEZE AND THAW)	FOOTINGS	25	N	
	SLAB ON GRADE	25	N	
	LEAN MIX	5	N	
	HOUSEKEEPING PADS	25	N	
EXTERIOR EXPOSED CONCRETE	FOUNDATION/RETAINING WALLS	25	F-2	
	SLAB ON GRADE, SIDEWALKS	32	C-2	
	FROST SLABS/APRON SLABS	35	C-1	
	APPARATUS BAY SLAB ON GRADE	35	C-1	
GROUT	MASONRY FILL/BOB BEAMS	25 (FINE GROUT)		
				CONFORM TO REQUIREMENTS OF CSA A179

1) STRENGTH SPECIFIED AT 28 DAYS U.N.O IN DRAWINGS AND SCHEDULES. FOR COLUMNS AND WALLS ONLY. FOR 60MPa AND 85MPa CONCRETE ARE PERMITTED TO BE 56 DAY MIX. 70MPa AND ABOVE CONCRETE ARE PERMITTED TO BE AT 90 DAYS MIX.
2) REINFORCED WITH SYNTHETIC FIBERS ADDED AT BATCHING PLANT - SEE SPECIFICATIONS

WHERE MECHANICAL SERVICE PIPES PASS THROUGH LOAD BEARING FOUNDATION WALLS, PROVIDE STEEL SLEEVES (MIN.500) LARGER THAN PIPE (TYPICAL)

LOWER ELEVATIONS AT UNDERSIDE OF COLUMN AND WALL FOOTINGS, WHERE REQUIRED, BUT LIMITED TO SUIT STORM / SANITARY, WATER / FIRE LINES AND ELECTRICAL DUCT BANKS. THE MAXIMUM SLOPE FROM THE PIPE EXCAVATION TO THE UNDERSIDE OF ADJACENT FOOTING ELEVATIONS SHALL NOT EXCEED 7 VERTICAL TO 10 HORIZONTAL.



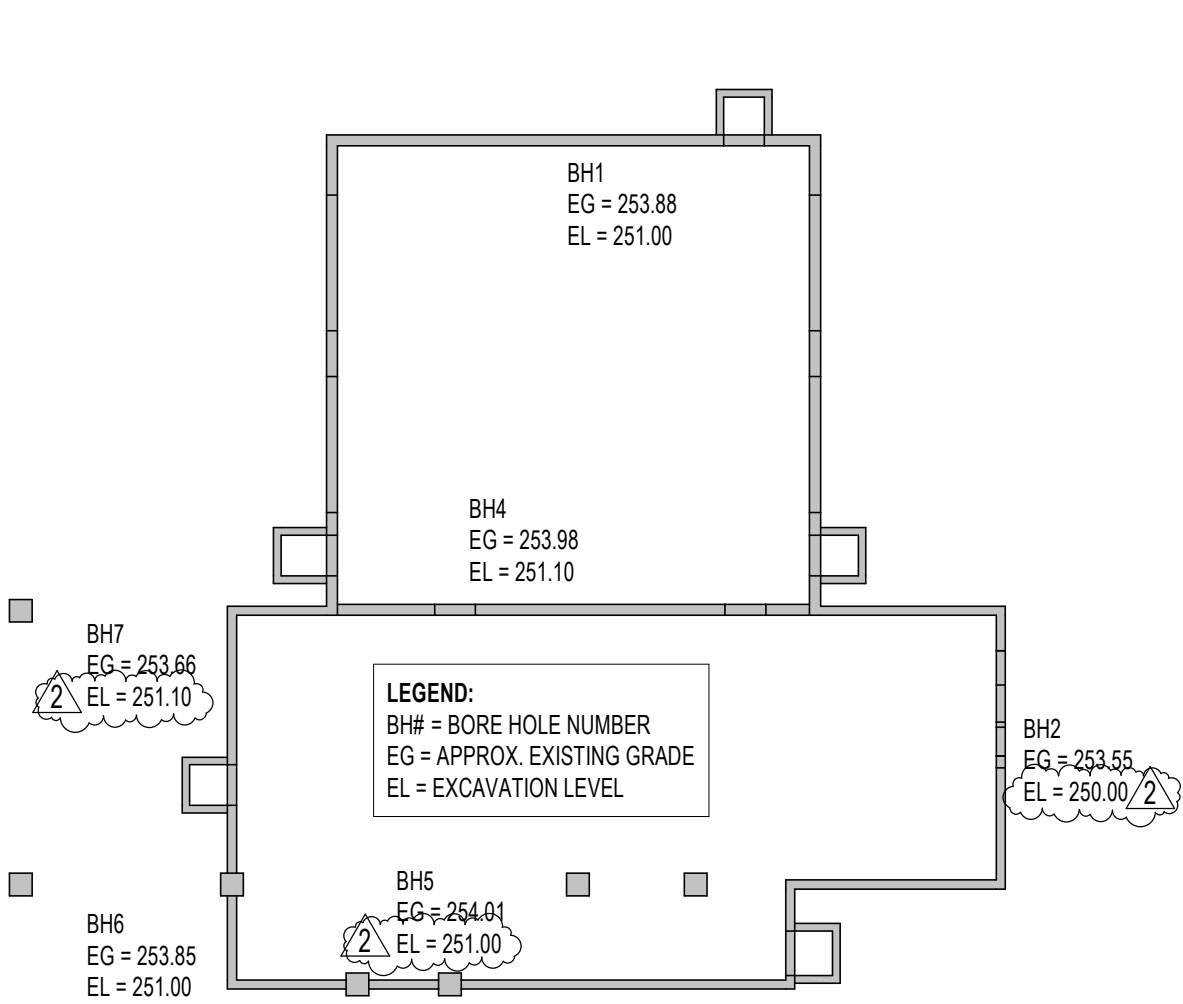
DIAGRAMMATIC SECTION THROUGH THE BUILDING
SHOWING PROPOSED ENGINEERED FILL N.T.S.
(REFER TO ENGINEERED FILL NOTES ON THIS DRAWING)

DESIGN CRITERIA NOTES

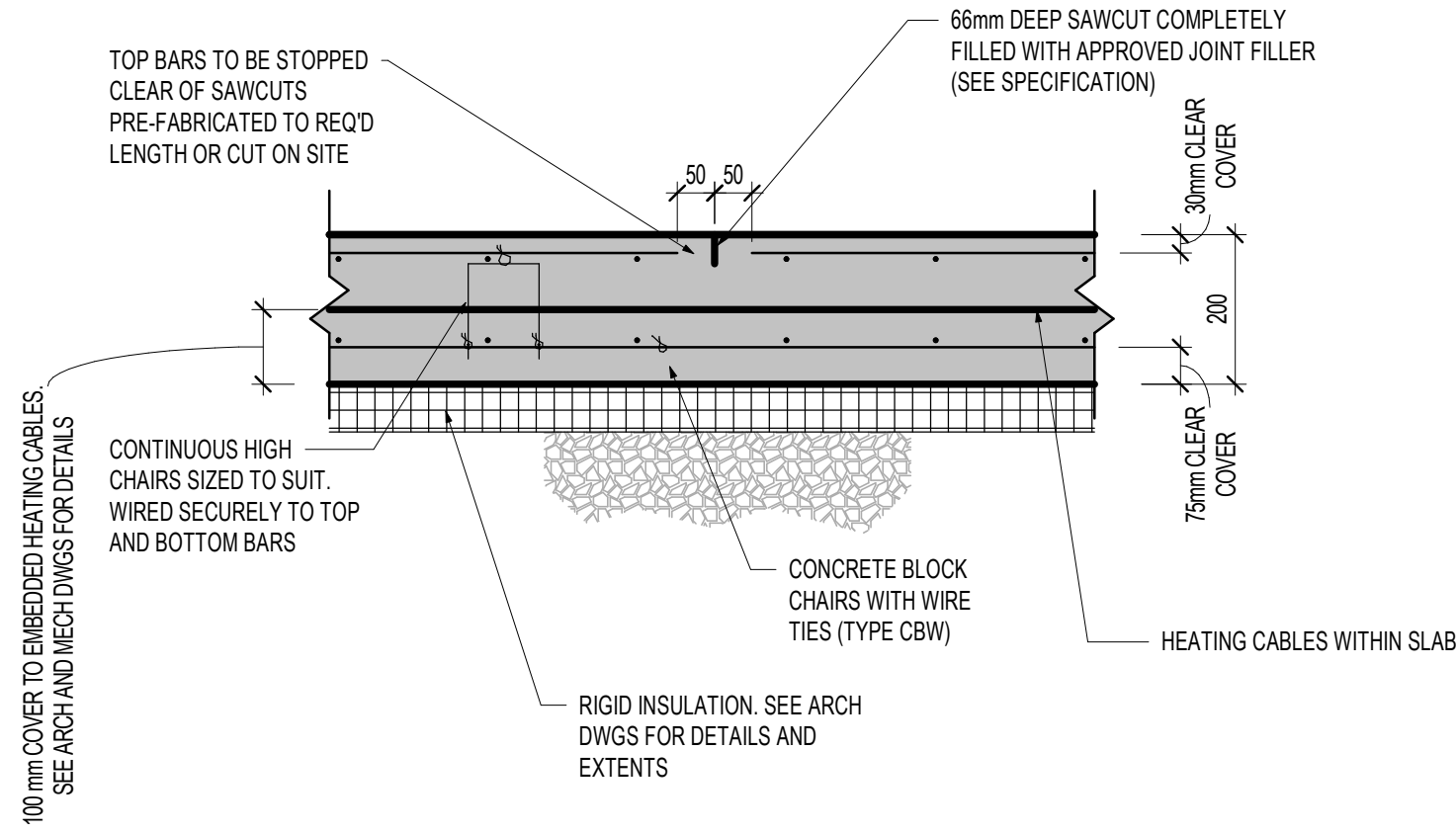
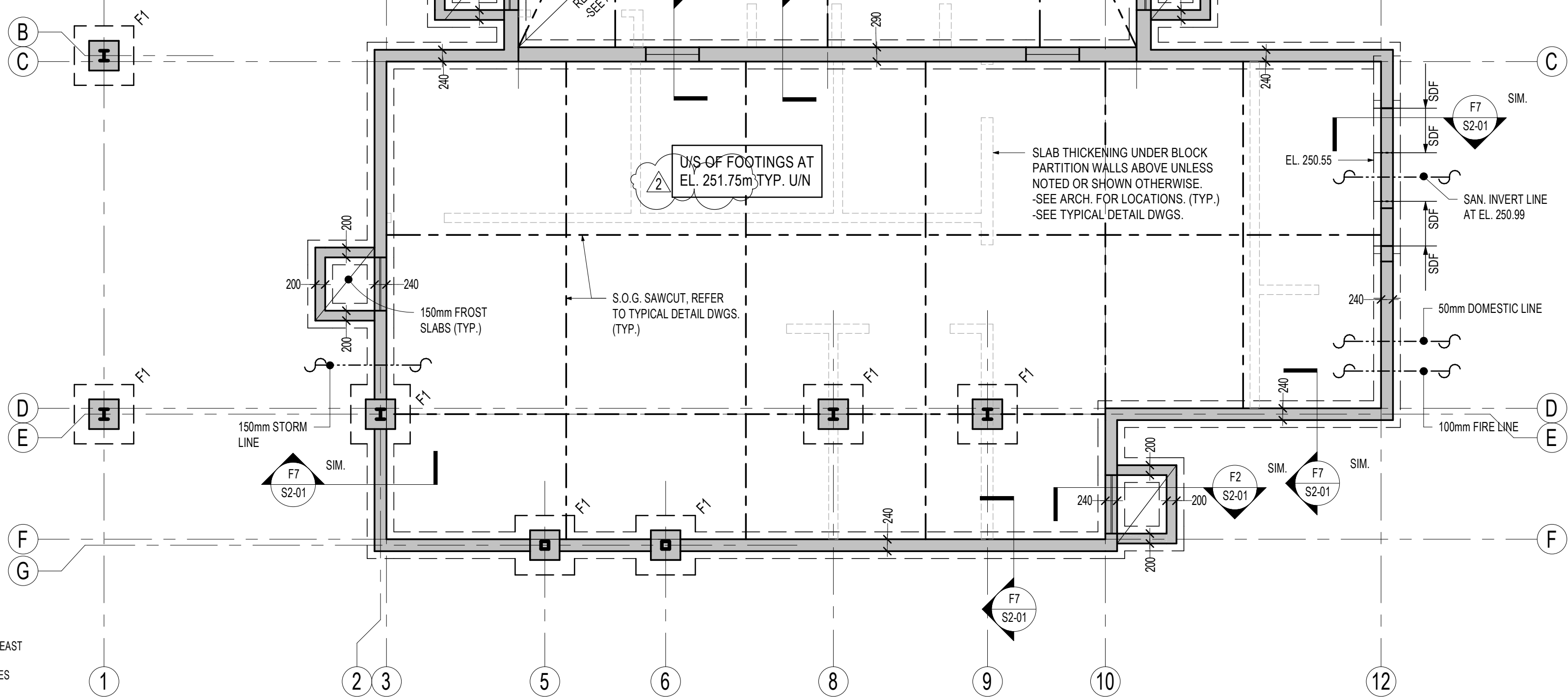
- GENERAL
 - THE PROJECT HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2012 OBC (O. REG. 332/12 AS AMENDED) INCLUDING CLAUSES 4.1.6.1(1), 4.1.6.4(3), 4.1.7 AND 4.1.8.
 - IT IS THE RESPONSIBILITY OF THE CONTRACTOR WHO IS SUPPLYING AND INSTALLING EQUIPMENT, THAT ALL ELEMENTS OF STRUCTURES LISTED IN TABLE 4.1.8.18 OF THE OBC 2012 ARE DESIGNED IN ACCORDANCE WITH CLAUSE 4.1.8.18.
 - BUILDING IMPORTANCE CATEGORY (SNOW, WIND, AND EARTHQUAKE) IS POST DISASTER.
 - STIFF ELEMENTS NOT PART OF SFRS SHALL BE SEPARATED FROM THE STRUCTURE AS PER OBC CLAUSE 4.1.8.3 (6a). EXAMPLES INCLUDE, BUT NOT LIMITED TO MASONRY PARTITIONS, BRICK VENEER, PRECAST CLADDING ETC. IT IS THE RESPONSIBILITY OF THE SUBCONTRACTOR TO PROVIDE SHOP DRAWINGS, STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER DEMONSTRATING COMPLIANCE. PROVIDE MINIMUM 15mm SEPARATION UNLESS NOTED OTHERWISE.
 - MISCELLANEOUS METAL, PRECAST AND STAIR FABRICATORS SHALL:
 - PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION; STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
 - DESIGN ALL GUARDS TO MEET LATERAL LOADS DESCRIBED IN OBC 4.1.5.14.
 - DESIGN ALL HANDRAILS TO MEET LOADS DESCRIBED IN OBC 3.4.6.5(2).
 - DESIGN ALL STAIRS TO SUPPORT A MINIMUM LIVE LOAD OF 4.8kPa.
 - ARCHITECTURAL PRECAST FABRICATOR SHALL:
 - PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION; STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
 - WHERE PRECAST IS USED AS A GUARD DESIGN THE PRECAST AND CONNECTIONS TO MEET LATERAL LOADS DESCRIBED IN OBC 4.1.5.14.
- LATERAL LOADS ON STRUCTURE
 - WIND $q(150) = 0.44kPa$ $C_e = (h/10)^{1/15}$ NOT LESS THAN 0.9. $C_d = 2.0$ $C_p =$ AS PER FIGURE 4.1.7.6-A OF NBC 2015
 - EARTHQUAKE $Sa(0.2) = 0.167$ $PGA = 0.105$ $F_a = 1.24$ $Sa(0.5) = 0.096$ $SITE CLASS = D$ $F_s = 1.55$ $Sa(1.0) = 0.053$ $R_d = 2.0$ $I_e = 1.5$ $Sa(2.0) = 0.026$ $R_o = 1.5$ $I_e F_a S_a(0.2) = 0.31$ $Sa(5.0) = 0.0065$ $Sa(10.0) = 0.0027$
SFRS CONSISTS MODERATELY DUCTILE MASONRY SHEAR WALLS AND SQUAT SHEAR WALLS.
METHOD OF ANALYSIS - STATIC
- FOUNDATION WALLS
 - WALLS RETAINING EARTH ARE DESIGNED TO SAFELY WITHSTAND HORIZONTAL EARTH PRESSURE
 $P = \gamma H (W_L h + q)$ $K = 0.4$ $W_L = 22kNm^3$ $q = 12kPa$ $h =$ DEPTH IN METRES
 - THE WALLS HAVE BEEN DESIGNED ASSUMING FREE DRAINING BACKFILL OR THE USE OF A DRAINAGE CORE TO PREVENT THE BUILD-UP OF HYDROSTATIC PRESSURE.

ENGINEER FILL NOTES

- GENERAL
 - THE FOLLOWING ARE MINIMUM REQUIREMENTS FOR PLACING ENGINEERED FILL WITHIN THE BOUNDARIES OF THE BUILDING ENVELOPE AND EXTENDING BEYOND PERIMETER OF THE BUILDING FOUNDATIONS BY A MIN. OF 3000mm AND SLOPING DOWNWARD TO THE SUB-GRADE, IN ALL DIRECTIONS, AT 45°.
 - PRIOR TO THE START OF CONSTRUCTION THE CONTRACTOR SHALL CONVEY A MEETING TO BE ATTENDED BY AT LEAST THE ARCHITECT, STEPHENSON ENGINEERING, THE SOIL CONSULTANT, THE GENERAL CONTRACTOR, AND THE EXCAVATION AND BACKFILLING CONTRACTOR. THE PURPOSE OF THIS MEETING IS TO ENSURE THAT ALL PARTIES UNDERSTAND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND TO DISCUSS PROCEDURES, TIMING, MATERIALS AND TESTING, ETC.
 - REFER ALSO TO THE SPECIFICATION, THE SOIL REPORT AND DIAGRAMMATIC SECTION.
- MATERIALS
 - ALL MATERIAL TO BE USED AS FILL MUST BE APPROVED EXISTING ON-SITE MATERIAL OR IMPORTED GRANULAR 'B' MATERIAL AS APPROVED BY THE SOIL CONSULTANT.
 - THE LAYER IMMEDIATELY BELOW THE SLAB-ON-GRADE SHALL BE A MIN. OF 250mm, 19mm CLEAR CRUSHED STONE, ONLY THE EXISTING ON-SITE MATERIAL WHICH ARE FREE OF TOP SOIL AND ARE NOT WET MAY BE SUITABLE FOR REUSE, AND ALL MATERIAL SHOULD BE REVIEWED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER DURING EXCAVATION TO DETERMINE THE SUITABILITY OF THE EXISTING FILL MATERIAL. ALL IMPORTED BORROW FILL MATERIAL FROM LOCAL SOURCES SHOULD BE FREE FROM ORGANIC MATERIAL AND FOREIGN OBJECTS, AND SHOULD BE TESTED GEOTECHNICALLY BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER PRIOR TO TRANSPORT TO THE SITE.
- EXECUTION
 - REMOVE AND DISPOSE OF ALL EXISTING ORGANIC MATERIAL, FILL, AND CONTAMINATED MATERIAL DOWN TO NATURAL UNDISTURBED, UN-CONTAMINATED SUB-GRADE AS INDICATED ON BULK EXCAVATION PLAN AND SOILS REPORT.
 - THE SUB-GRADE SHALL BE PROOF ROLLED WITH HEAVY ROLLER (NO VIBRATION) TO MIN. 98% STANDARD PROCTOR MAXIMUM DRY DENSITY.
 - ANY LOOSE OR SOFT SPOT SHALL BE SUB-EXCAVATED AND BACKFILLED WITH APPROVED COMPACTED MATERIAL.
 - FILL REQUIRED TO RAISE THE GRADES SHALL COMPRISE OF APPROVED ON-SITE AND IMPORTED GRANULAR 'B' MATERIAL PLACED IN SUCCESSIVE 300mm LAYERS EACH COMPACTED TO AT LEAST 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.
 - THE LAYER IMMEDIATELY BELOW THE SLAB-ON-GRADE SHOULD BE A 250mm MIN. LAYER OF 19mm CLEAR STONE ROLLED AND COMPACT.
 - ALL PROCEDURES, EQUIPMENT AND MATERIALS SHALL BE APPROVED BY THE SOIL CONSULTANT WHO SHALL BE ENGAGED "FULL TIME" TO SUPERVISE THIS WORK.
 - CONDITIONS AS OUTLINED IN THE CONTRACT DOCUMENTS ARE ASSUMED AND ARE BASED UPON INFORMATION AVAILABLE AT THE TIME THAT THE DOCUMENTS WERE PREPARED.
 - THE SOIL CONSULTANT SHALL ISSUE, VIA "E-MAIL", DAILY REPORTS OF THE WORK.
 - IF ANY ASPECT OF THE ACTUAL WORK IS NOT AS ASSUMED, THEN THE SOIL CONSULTANT SHALL ADVISE THE ARCHITECT IMMEDIATELY, BY TELEPHONE, BEFORE PROCEEDING.
 - NOTE THAT ONLY THE EXISTING ON-SITE MATERIAL, AS NOTED MAY BE SUITABLE FOR REUSE FOR BACKFILLING OF TRENCHES, ETC., OR AGAINST FOUNDATION WALLS.
 - FOR AREAS UNDER DRIVEWAYS AND PARKING ETC., OUTSIDE BUILDING ENVELOPE, REFER TO SPECIFICATION AND SOIL REPORT.



BULK EXCAVATION PLAN
1 : 200



- NOTES:
- MAXIMUM SPACING OF BOTTOM AND TOP CHAIRS 1200 o/c.
 - FOR JOINTS IN OTHER SLABS-ON-GRADE, SEE TYPICAL DETAILS.

SL1
S1-01
1 : 10

FOUNDATION PLAN

1 : 75

FOUNDATION PLAN NOTES

- TOP OF SLAB - ON - GRADE TO BE 0.0 BELOW FINISHED FLOOR DATUM ELEVATION 253.15m, EXCEPT AS NOTED. TOS = TOP OF SLAB.
- FOOTINGS SHALL BEAR ON ENGINEERED FILL DEVELOPED OVER COMPETENT NATIVE SOILS CAPABLE OF SUSTAINING A MINIMUM OF 150 kPa (SL) AND 225 kPa (ULS).
- REFER TO THE SOIL REPORT No. GOR-0024781-10 DATED AUG. 03, 2018 PREPARED BY EXP SERVICES INC.
- SOIL AT THE UNDERSIDE OF THE FOOTINGS IS TO BE INSPECTED AND APPROVED BY A REPRESENTATIVE OF A SOILS CONSULTANT BEFORE PLACING CONCRETE.
- REFER ALSO TO SITE PREPARATION NOTES ON THIS DRAWING.
- CO-ORDINATE ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES TO ENGINEER PRIOR TO PROCEEDING WITH ANY WORK.
- UNDERSIDE OF WALL FOOTINGS TO BE AT ELEVATIONS AS NOTED ON PLAN.
- SDF = STEP DOWN FOOTING.
- UNLESS OTHERWISE SHOWN, ALL WALL FOOTINGS TO BE 300mm DEEP WITH 150mm PROJECTIONS EACH SIDE.
- FILL REQUIRED ON BOTH SIDES OF FOUNDATION WALLS SHALL BE PLACED AND COMPACTED SIMULTANEOUSLY ON EACH SIDE TO EQUALIZE SOIL PRESSURE.
- PROVIDE SLAB DEPRESSIONS AND SLOPES, OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS, AS REQUIRED BY THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
- THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE INCLUDING STRIP FOOTINGS.
- GENERAL SLAB - ON - GRADE IS 100mm THICK REINFORCED WITH SYNTHETIC FIBRES (REFER TO CONCRETE SPECIFICATION), EXCEPT AS NOTED.
- CONCRETE STRENGTHS - SEE CONCRETE MIX SCHEDULE.
- SEE TYPICAL NOTES, TYPICAL DETAILS, AND ALL OTHER DRAWINGS.

FOOTING SCHEDULE				
FOOTING NUMBER	FOOTING LENGTH	FOOTING WIDTH	FOOTING THICKNESS	FOOTING REINF. B.E.W.
F1	1200	1200	300	4-15M

THE CONTENTS OF THIS DRAWING AND SPECIFICATIONS
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ISSUE OR REVISION

NO.	ISSUED FOR	DATE
1	ISSUED FOR 90% SUBMISSION	NOV17/2020
2	ISSUED FOR ADDENDUM #1	JAN13/2026

YORK REGION PRS #33
RFTC 379-21

PROJECT :

CLIENT



Salas
O'Brien

2235 Sheppard Ave. E.
Toronto, ON

Suite No. 1100
M2J 5B5

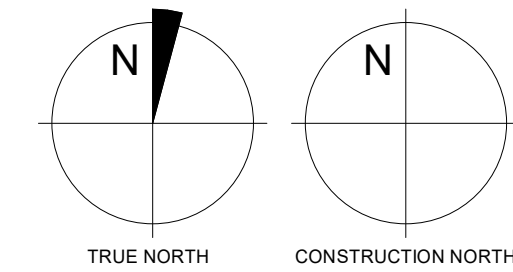
PROFESSIONAL SEAL



DWG TITLE

FOUNDATION PLAN

ORIENTATION



DATE

JAN. 2026

SCALE

As indicated

DRAWN BY

AE

CHECKED BY

JG

DWG STATUS

ADDENDUM #1

PROJECT NO.

20190540

DRAWING NO.

S1-01

REVISION

2

1/13/2026 12:38 PM

ISSUE OR REVISION

NO.	ISSUED FOR	DATE
1	ISSUED FOR 90% SUBMISSION	NOV17/2020
2	ISSUED FOR ADDENDUM #1	JAN13/2026

YORK REGION PRS #33
RFTC 379-21

2960 TESTON ROAD, VAUGHAN

PROJECT :
CLIENT :



Salas O'Brien
2235 Sheppard Ave. E.
Toronto, ON
Suite No. 1100
M2J 5B5

PROFESSIONAL SEAL



DWG TITLE

FOUNDATION
SECTIONS

ORIENTATION

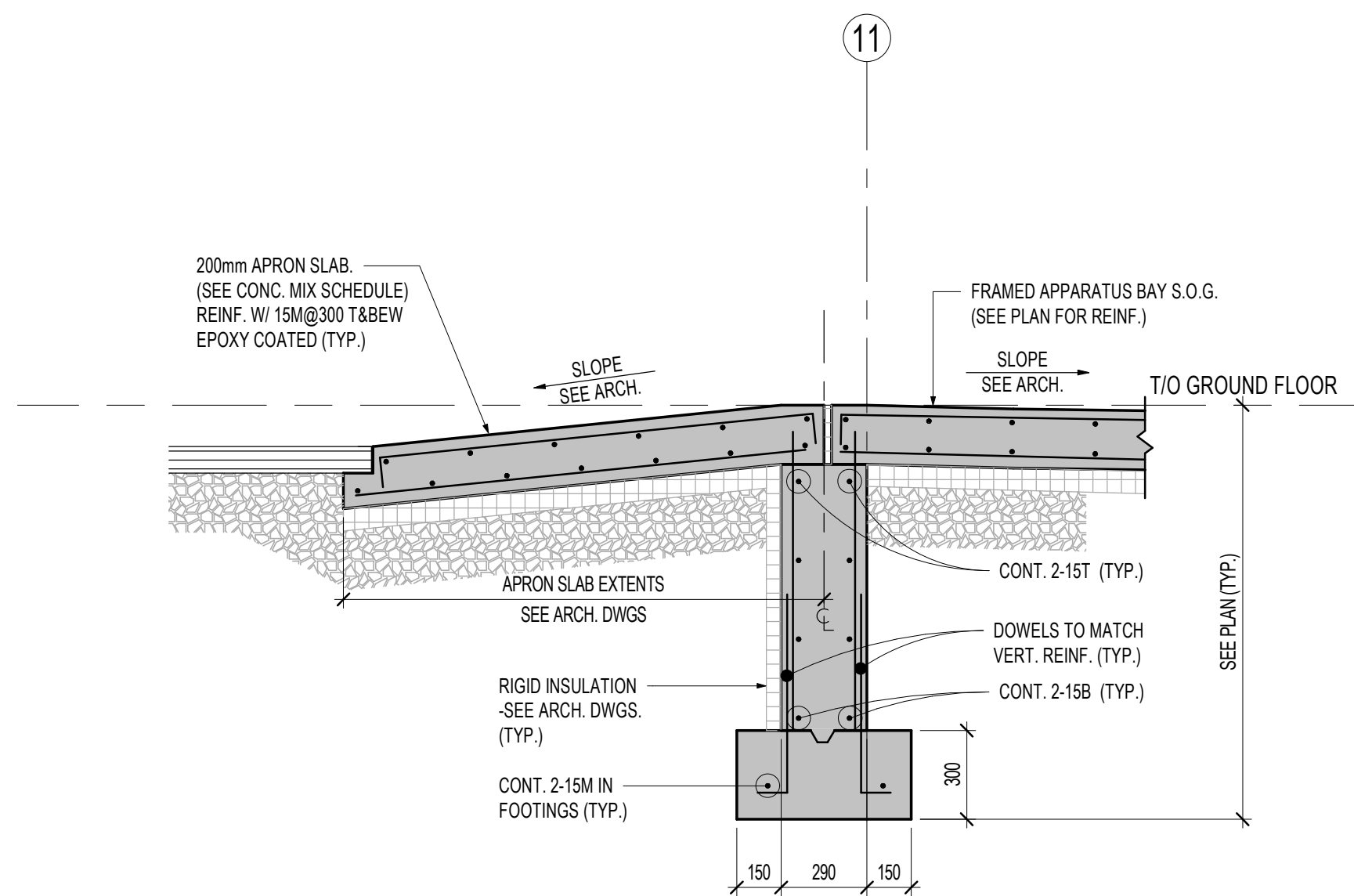
DATE JAN. 2026

SCALE As indicated
DRAWN BY AE
CHECKED BY JG

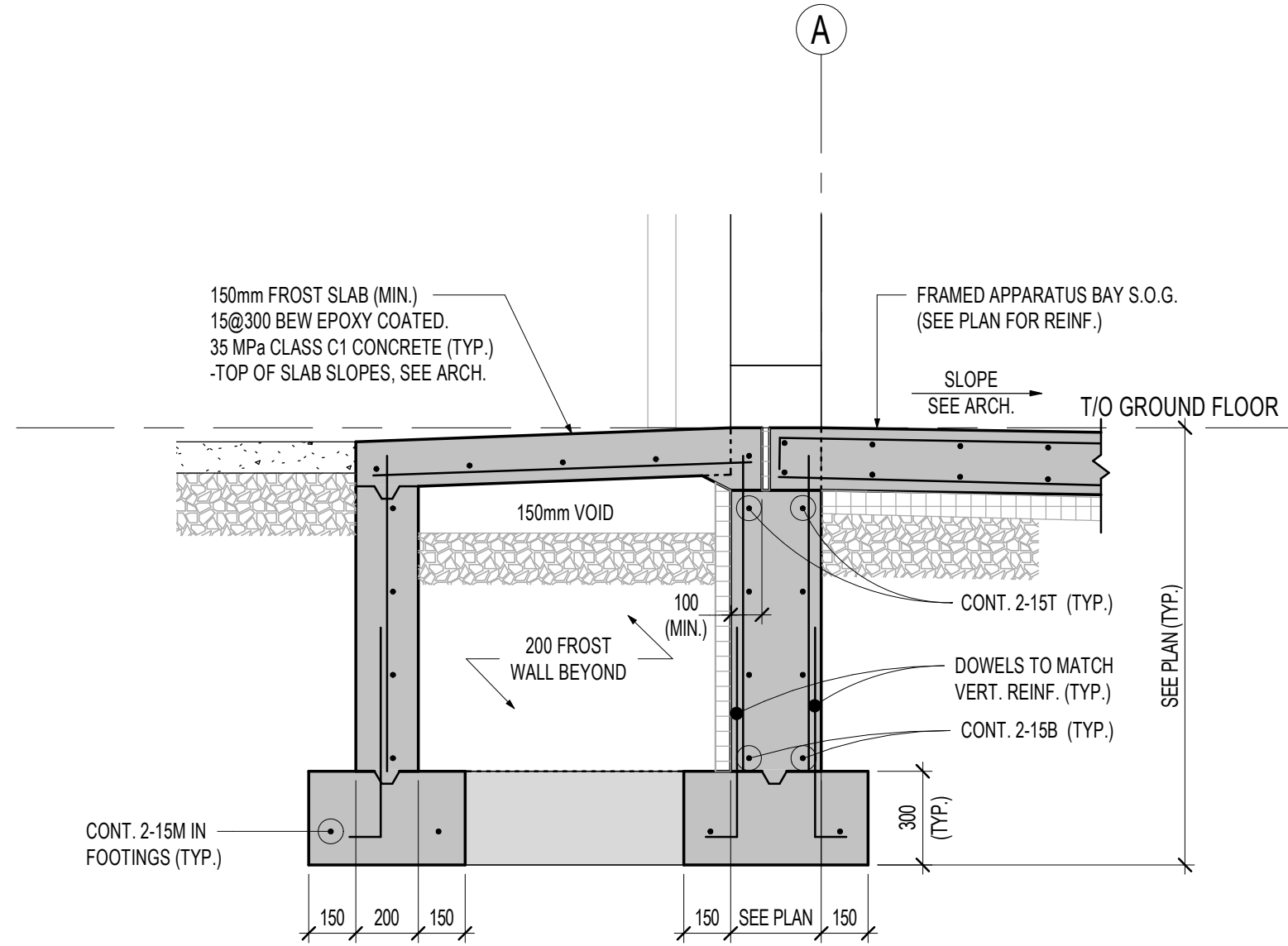
ADDENDUM #1

PROJECT NO. 20190540

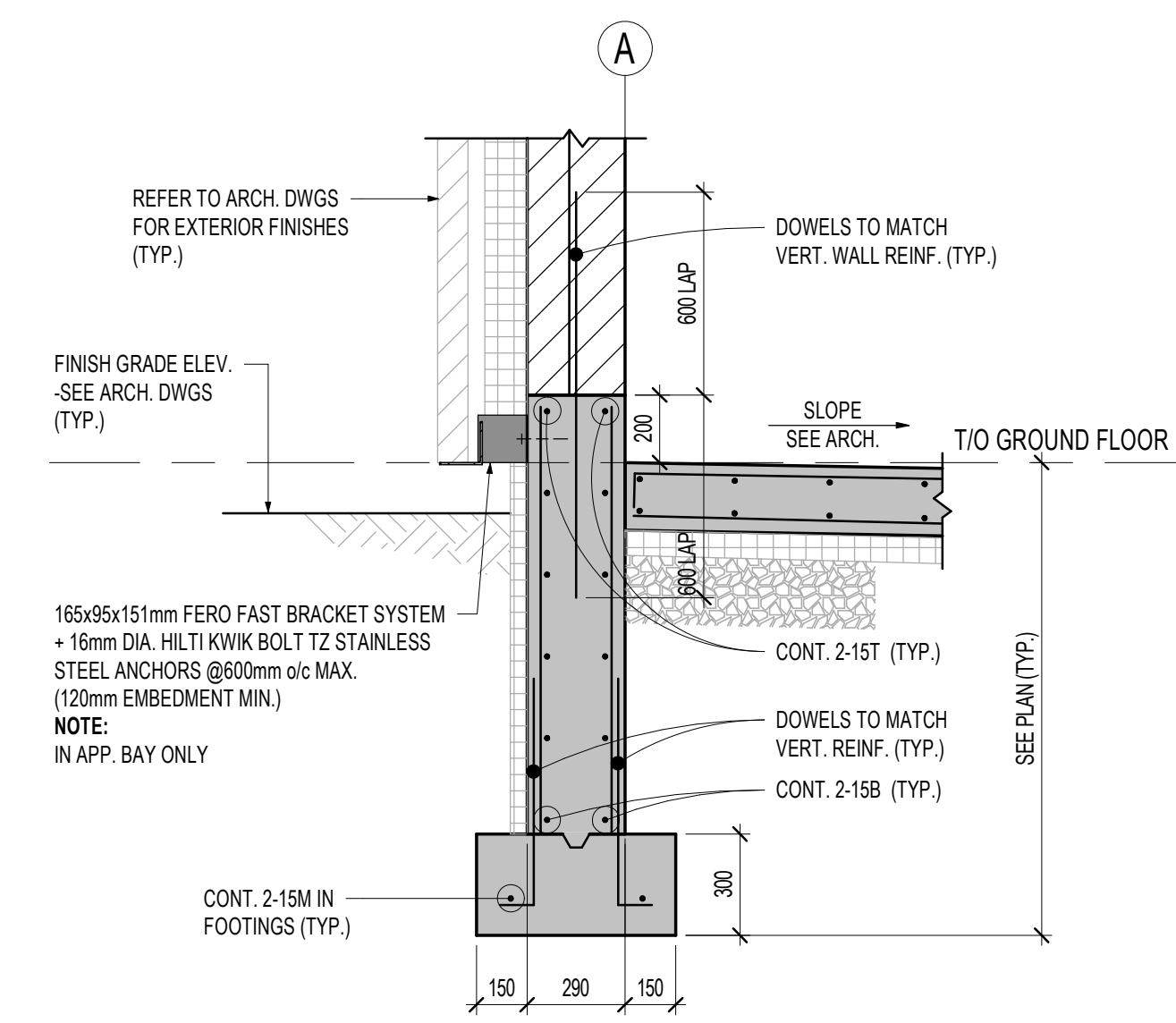
DRAWING NO. S2-01
REVISION 2



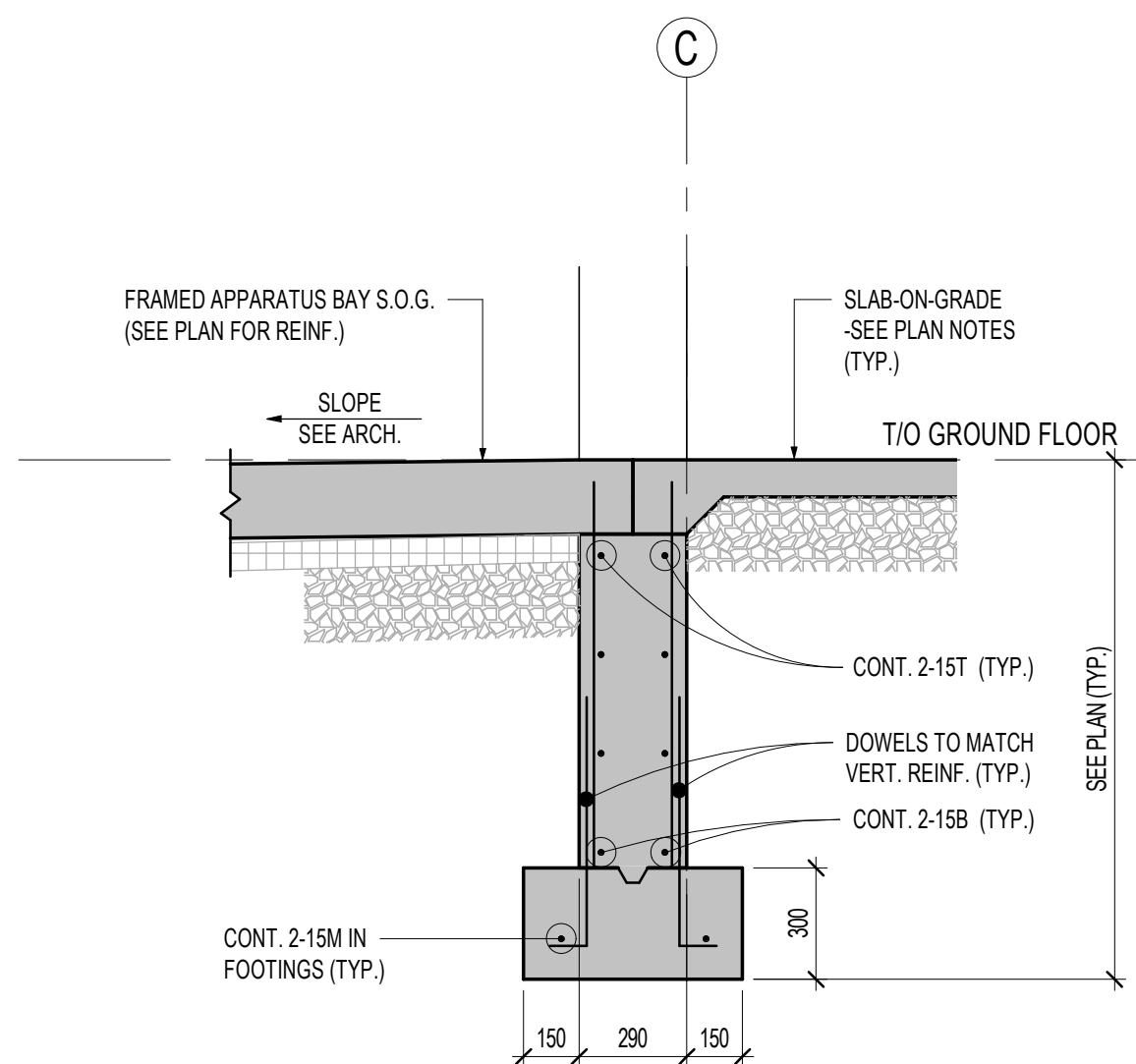
F1 SECTION
S2-01 1:20



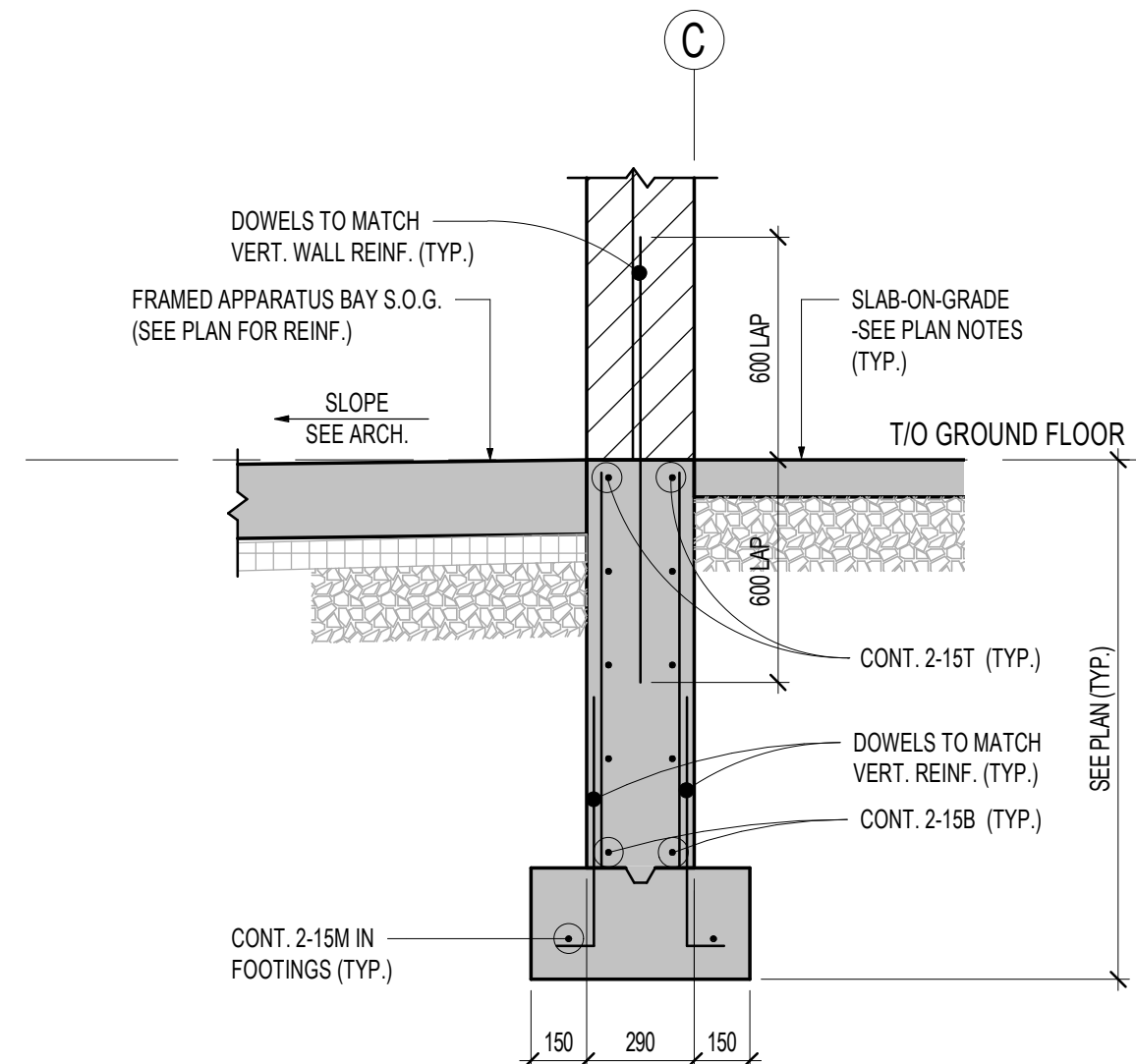
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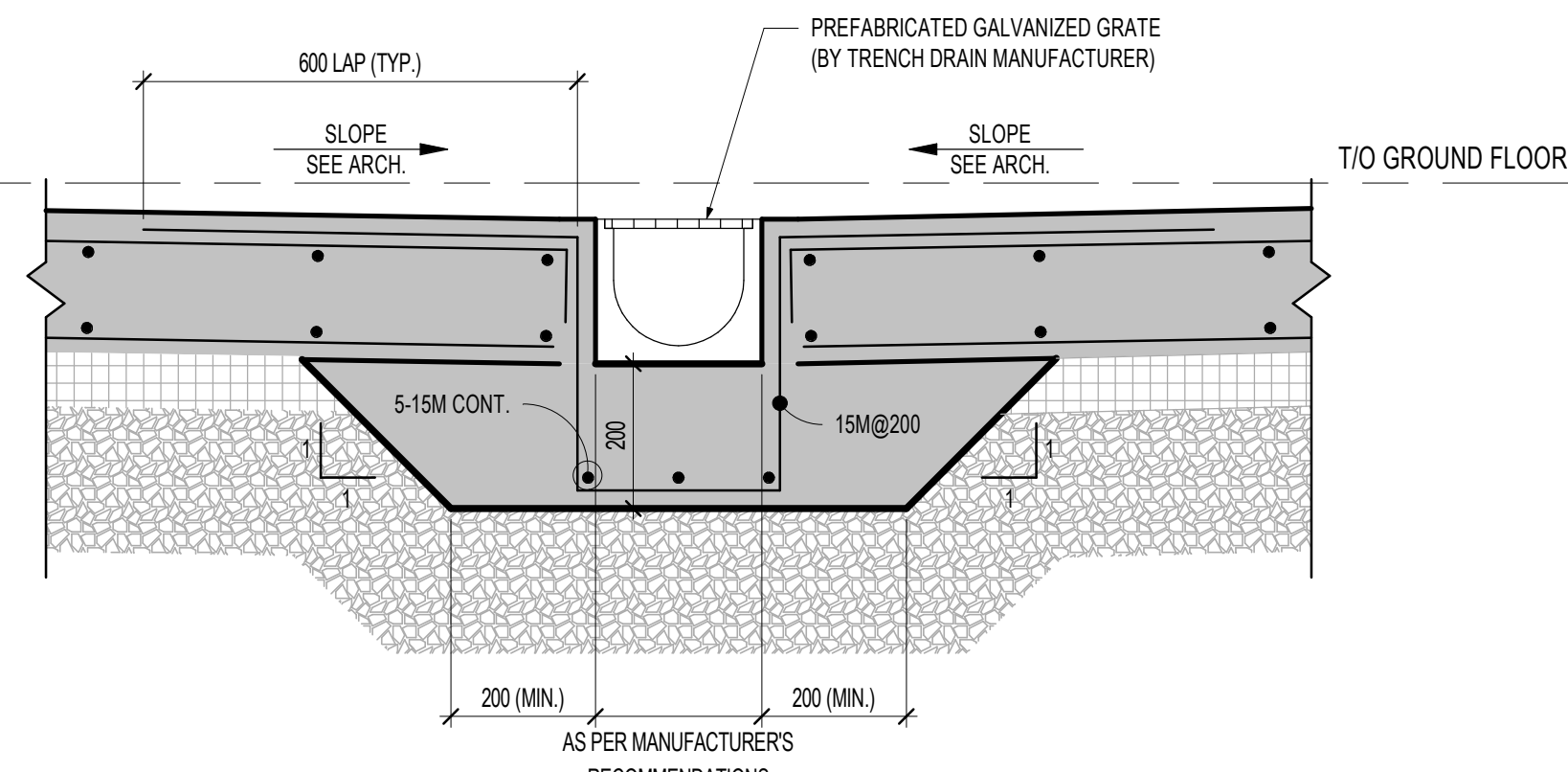
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S2-01 1:20



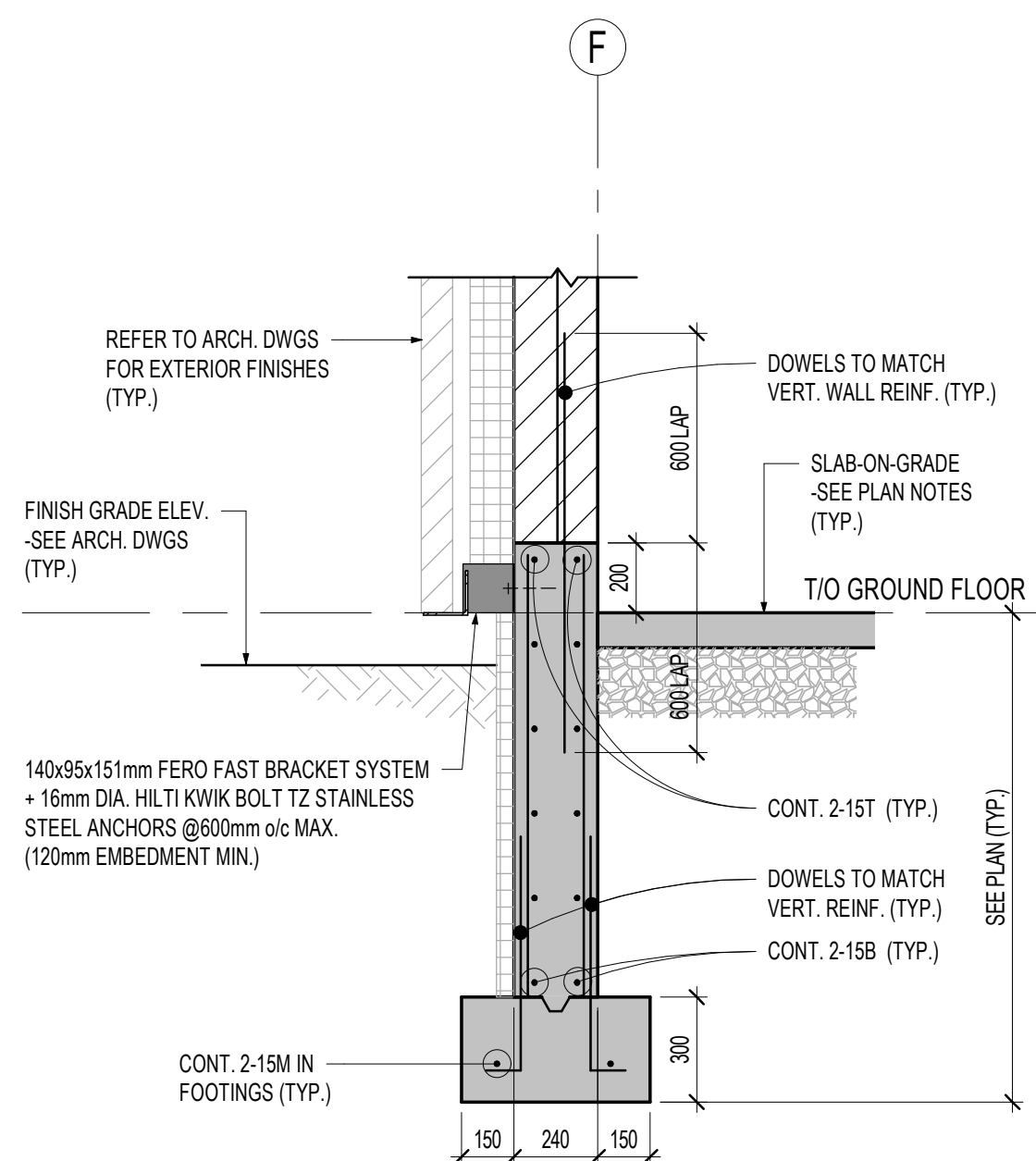
F4 SECTION
S2-01 1:20



F5 SECTION
S2-01 1:20



F6 SECTION
S2-01 1:10



F7 SECTION
S2-01 1:20

NOTE:
TYPICAL FOUNDATION WALL REINFORCING
(UNLESS NOTED OTHERWISE ON SECTIONS
OR SHEAR WALL ELEVATIONS)
10M @460 VEF
10M @320 HEF

FOR 190mm/200mm WALLS:
10M @320 VERT. CENTRE OF WALL
10M @200 HORIZ. CENTRE OF WALL

STEEL COLUMN SCHEDULE						
U/S LOW ROOF DECK						
	W200x36	W200x36	W200x36	W200x36	W200x36	HSS 152x152x6.4
GROUND FLOOR						
U/S B.P.L. -350 (U.N.O.)						
BASE PLATE SIZE ANCHOR RODS	375x25x375 (4)-AR1	375x25x375 (4)-AR1	375x25x375 (4)-AR1	375x25x375 (4)-AR1	375x25x375 (4)-AR1	375x25x375 (4)-AR1
PIER SIZE VERTICAL REINF. TIES	600x600 10-15V 10@300T	600x600 10-15V 10@300T	600x600 10-15V 10@300T	600x600 10-15V 10@300T	600x600 10-15V 10@300T	600x600 10-15V 10@300T
Column Locations	B-1	E-1	E-2	E-8	E-9	G-5 G-6

STEEL COLUMN SCHEDULE NOTES:

- FOR GRADE OF STRUCTURAL STEEL SEE GENERAL NOTES AND SPECIFICATION.
- LOADS FOR COLUMNS REPRESENT THE FACTORED LOAD IN KILONEWTONS APPLIED AT THE BASE OF THE COLUMN AND DO NOT INCLUDE THE WEIGHT OF THE FOUNDATION.
- BASE PLATE AND / OR CAP PLATE DIMENSION GIVEN LAST TO BE PARALLEL WITH COLUMN WEB.
- REFER ALSO TO TYPICAL NOTES AND DETAIL DRAWINGS.
- REFER TO STEEL COLUMN SCHEDULE FOR ANCHOR RODS AND FOR COLUMN BASE PLATE SIZES
- FOR ALL COLUMNS ABUTTING MASONRY, PROVIDE ADJUSTABLE MASONRY ANCHORS AS PER TYPICAL DETAIL. SEE TYPICAL DETAIL DRAWINGS.

ERECTION TOLERANCES FOR STEEL BEAMS

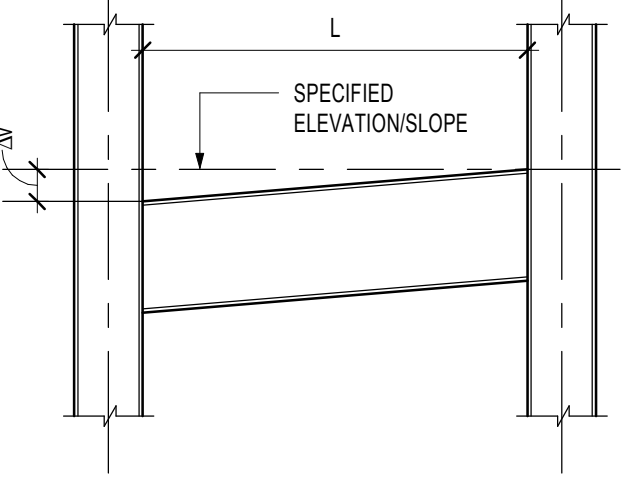
SB02A

(READ IN CONJUNCTION WITH SB02B)

1. VERTICAL DEVIATION FROM SPECIFIED ELEVATIONSLOPE.

FLOOR BEAMS: $\Delta v = \pm 10\text{mm}$ (3/8") OR = L/500

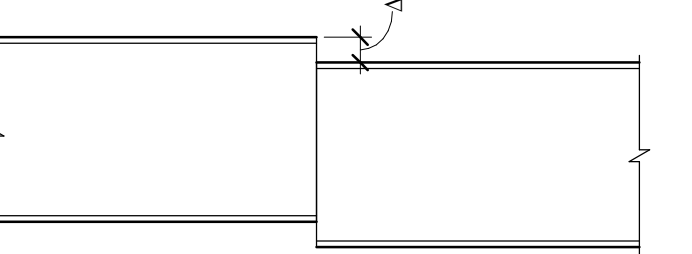
MEMBERS WITH ADJUSTABLE CONNECTIONS: $\Delta v = \pm 6\text{mm}$ (1/4") OR = L/1000



2. VERTICAL DEVIATION FROM SPECIFIED ELEVATION - ADJOINING MEMBERS

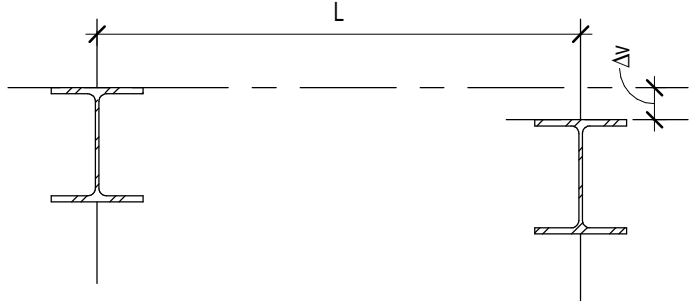
FLOOR BEAMS: $\Delta v = \pm 6\text{mm}$ (1/4")

MEMBERS WITH ADJUSTABLE CONNECTIONS: $\Delta v = \pm 2\text{mm}$ (3/32")



3. VERTICAL DEVIATION FROM ADJACENT BEAMS

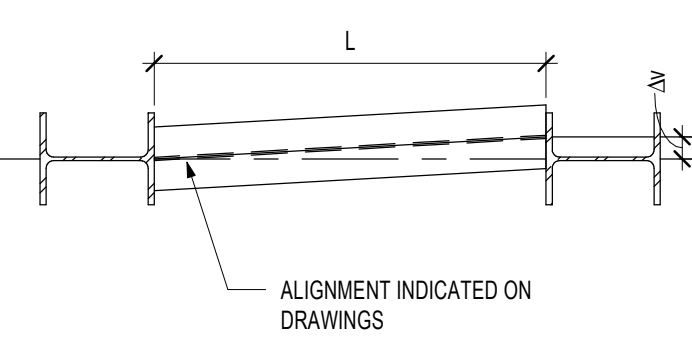
FLOOR BEAMS: $\Delta v = L/1000$



4. HORIZONTAL DEVIATION FROM INDICATED POSITION

FLOOR BEAMS: $\Delta H = \pm 12\text{mm}$ (1/2") OR L/500

SPANDREL BEAMS: $\Delta H = \pm 6\text{mm}$ (1/4") OR L/1000



NOTES

1. TOLERANCES PROVIDED IN THE DETAIL ABOVE SHALL NOT SUPERSEDE THE VALUES INDICATED IN CSA S16 AND REFERENCED DOCUMENTS.

ERECTION TOLERANCES FOR STEEL BEAMS

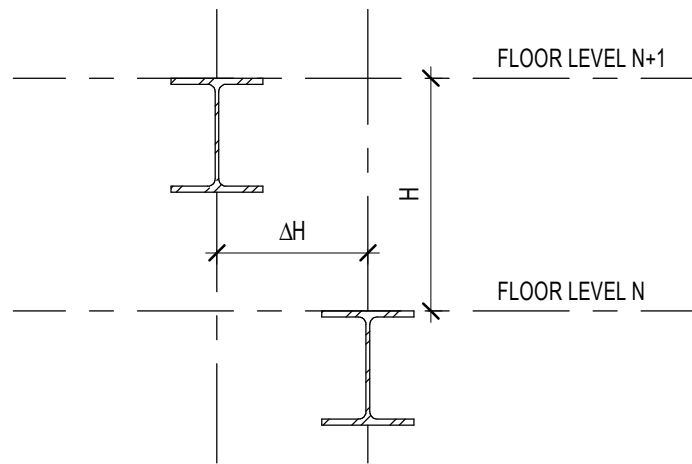
SB02B

(READ IN CONJUNCTION WITH SB02A)

5. HORIZONTAL DEVIATION FROM ADJACENT BEAMS

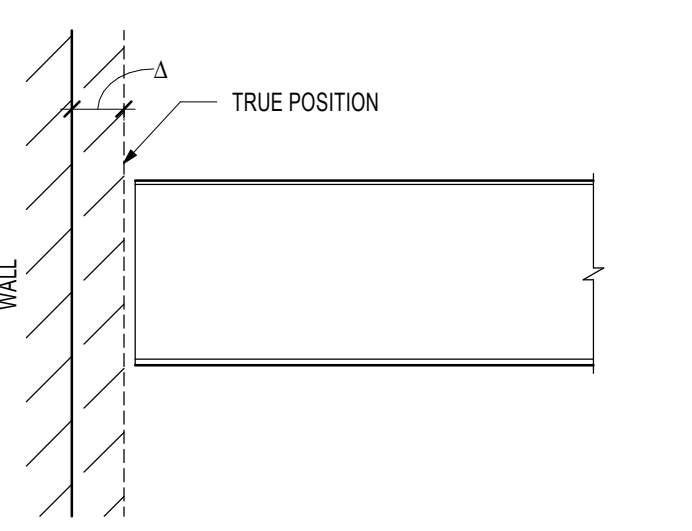
FOR $H < 3000\text{mm}$ (10'-0"): $\Delta H = \pm 5\text{mm}$ (3/16")

FOR $H > 3000\text{mm}$ (10'-0"): $\Delta H = H/600$



6. HORIZONTAL DEVIATION FROM SUPPORT POINT AT VERTICAL WALL

$\Delta = \pm 25\text{mm}$ (1")



NOTES

1. TOLERANCES PROVIDED IN THE DETAIL ABOVE SHALL NOT SUPERSEDE THE VALUES INDICATED IN CSA S16 AND REFERENCED DOCUMENTS.

2. FOR ERECTION TOLERANCES OF SPECIAL MEMBERS SUCH AS CRANE GIRDERS, CRANE RAILS AND MONORAIL BEAMS, SEE THE APPROPRIATE CODE RECOMMENDATIONS.

3. DEVIATIONS SHOWN FOR W-SHAPES ALSO APPLY TO BUILT-UP SECTIONS, HOLLOW STRUCTURAL SECTIONS, CHANNEL AND ANGLE SHAPES.

4. ERECTION TOLERANCES ARE TO BE MEASURED IN CALM WEATHER. RECORD AMBIENT TEMPERATURE AT TIME TOLERANCES ARE VERIFIED.

ERECTION TOLERANCES FOR STEEL COLUMNS

SC01A

(READ IN CONJUNCTION WITH SC01B, SB02)

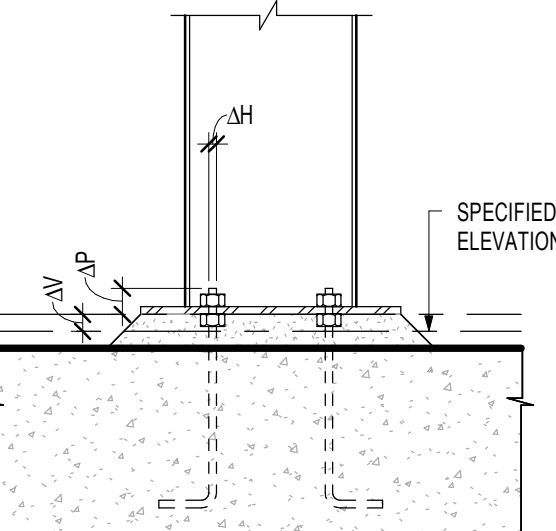
1. VERTICAL DEVIATION FROM SPECIFIED ELEVATION.

ANCHOR BOLTS: $\Delta P = + 30\text{mm}$ (1-3/16") = - 5mm (3/16")

$\Delta H = 3\text{mm}$ (1/8")

BASE PLATE: $\Delta V = \pm 5\text{mm}$ (3/16") 'SIMPLE CONSTRUCTION

$= \pm 3\text{mm}$ (1/8") 'CONTINUOUS CONSTRUCTION

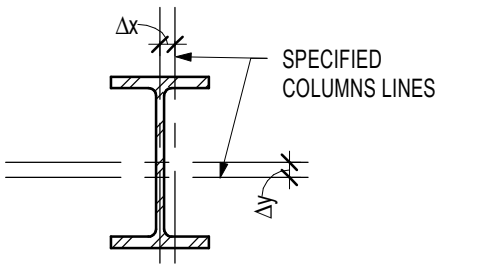


NOTE:

1. SEE TYPICAL DETAIL SAB02 FOR ADDITIONAL INFORMATION.

2. HORIZONTAL DEVIATION FROM SPECIFIED POSITION.

AT COLUMN BASE: $\Delta x/\Delta y = \pm 5\text{mm}$ (3/16")

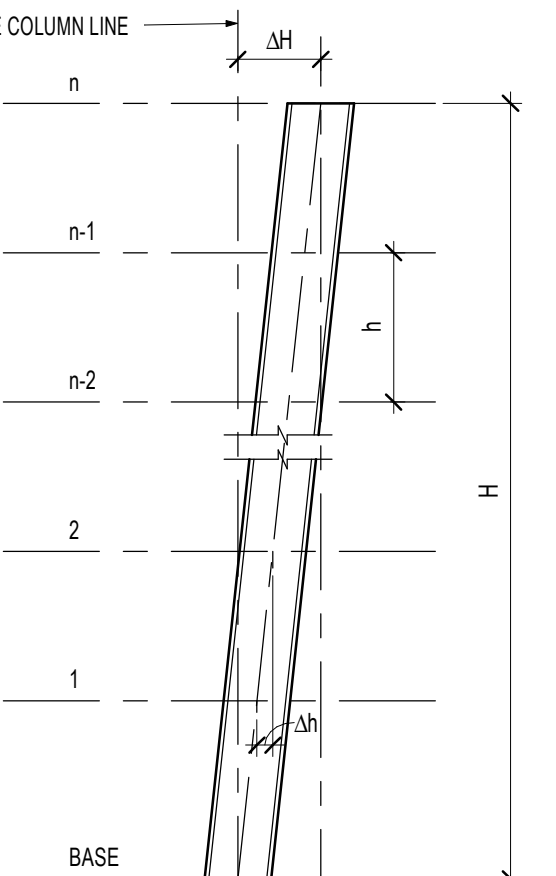


ABOVE COLUMN BASE:

EXTERIOR COLUMN: $\Delta H < H/1000$ TOTAL

COLUMN ADJACENT TO ELEVATOR SHAFTS: $\Delta H \leq \pm 25\text{mm}$ (1") TOTAL, $\Delta h < 2\text{mm}$ (3/32") STOREY

ALL OTHER COLUMNS: $\Delta H < H/500$ AND, $\Delta h \leq \pm 50\text{mm}$ (2") TOTAL, $\Delta h < 4\text{mm}$ (3/16") STOREY



NOTES

1. TOLERANCES PROVIDED IN THE DETAIL ABOVE SHALL NOT SUPERSEDE THE VALUES INDICATED IN CSA S16 AND REFERENCED DOCUMENTS.

ERECTION TOLERANCES FOR STEEL COLUMNS

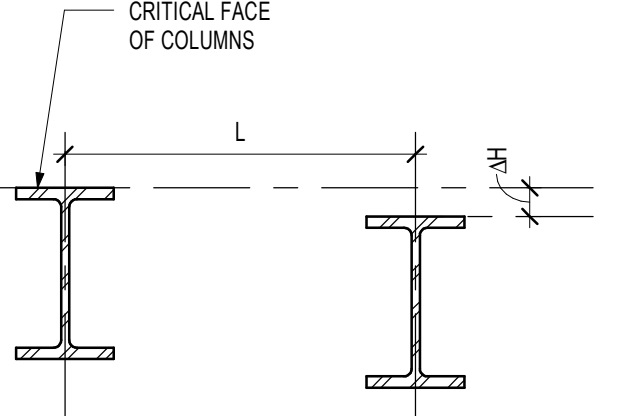
SC01B

(READ IN CONJUNCTION WITH SC01A)

3. HORIZONTAL DEVIATION FROM ADJACENT COLUMNS.

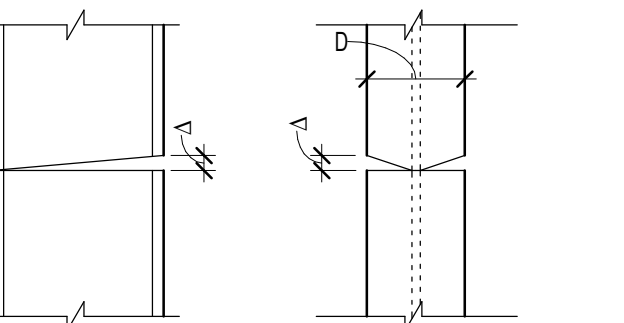
BASE LEVEL OR SPLICE LEVEL: OR = L/1000

$\Delta H = 10\text{mm}$ (3/8")



4. GAP BETWEEN BEARING SURFACES.

$\Delta \text{MAX} = 6\text{mm}$ (1/4")



NOTES

1. TOLERANCES PROVIDED IN THE DETAIL ABOVE SHALL NOT SUPERSEDE THE VALUES INDICATED IN CSA S16 AND REFERENCED DOCUMENTS.

2. FOR ERECTION TOLERANCES OF SPECIAL MEMBERS SUCH AS CRANE GIRDERS, CRANE RAILS AND MONORAIL BEAMS, SEE THE APPROPRIATE CODE RECOMMENDATIONS.

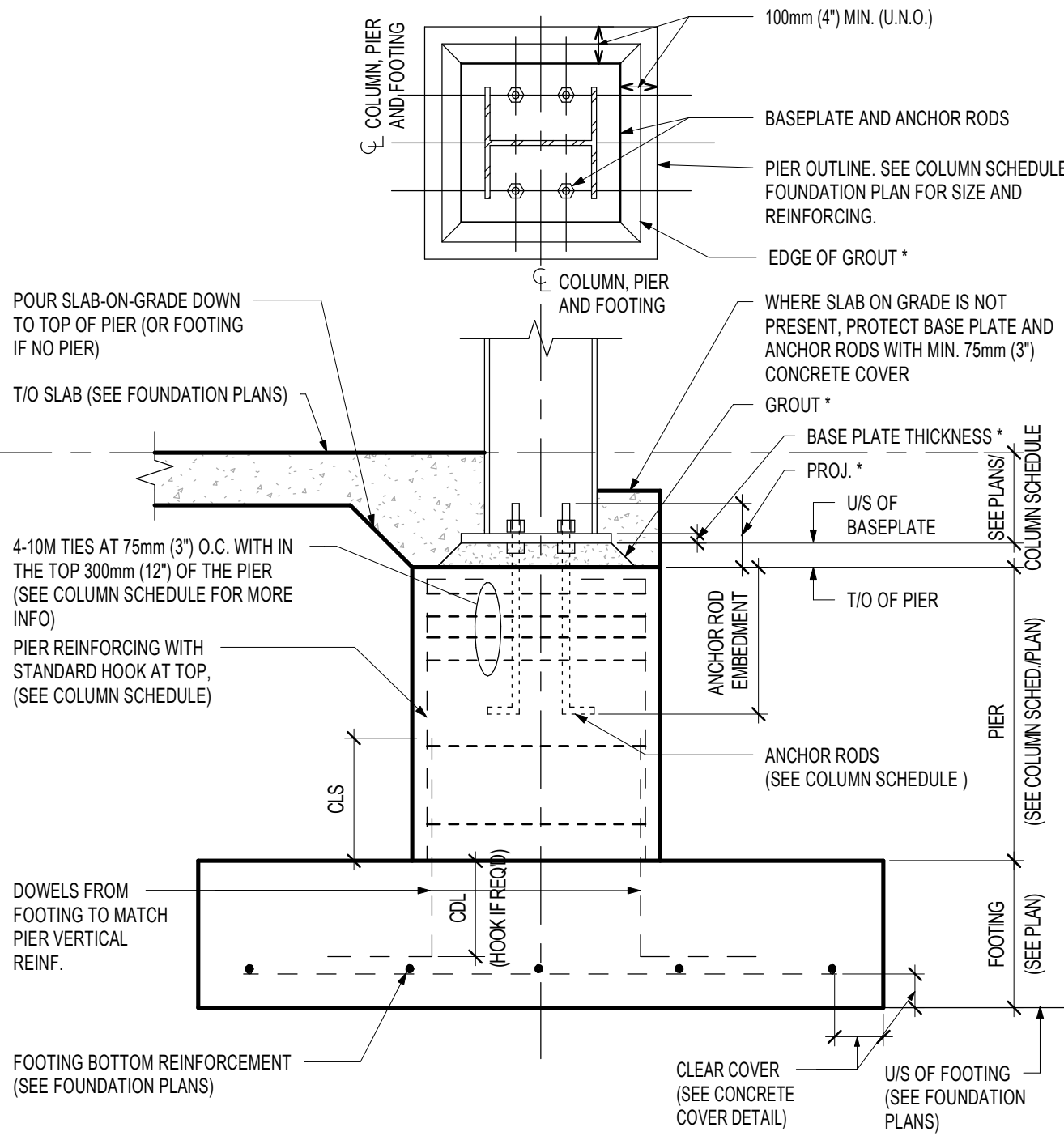
3. DEVIATIONS SHOWN FOR W-SHAPES ALSO APPLY TO BUILT-UP SECTIONS, HOLLOW STRUCTURAL SECTIONS, CHANNEL AND ANGLE SHAPES.

4. ERECTION TOLERANCES ARE TO BE MEASURED IN CALM WEATHER. RECORD AMBIENT TEMPERATURE AT TIME TOLERANCES ARE VERIFIED.

STEEL GRAVITY COLUMN BASE DETAIL

SC02

(READ IN CONJUNCTION WITH ANCHOR ROD SCHEDULE IN TYPICAL DETAIL SAB02)



NOTES:

1. FOOTINGS SHALL BE PLACED ON UNDISTURBED SOIL WITH A MINIMUM BEARING CAPACITY AS NOTED ON THE DRAWINGS.

2. GROUT UNDER BASE PLATES SHALL BE AN APPROVED PROPRIETARY BRAND PRE-MIXED, NON-METALLIC, NON-SHRINK GROUT UNLESS OTHERWISE APPROVED.

3. LEVELING PLATES ARE NOT PERMITTED.

4. REFER TO COLUMN SCHEDULE/FOUNDATION PLAN FOR BASE PLATES, ANCHOR ROD, PIER, FOOTING DIMENSIONS AND REINFORCEMENT.

5. REFER TO A04, SAB02 FOR ANCHOR ROD GRADE

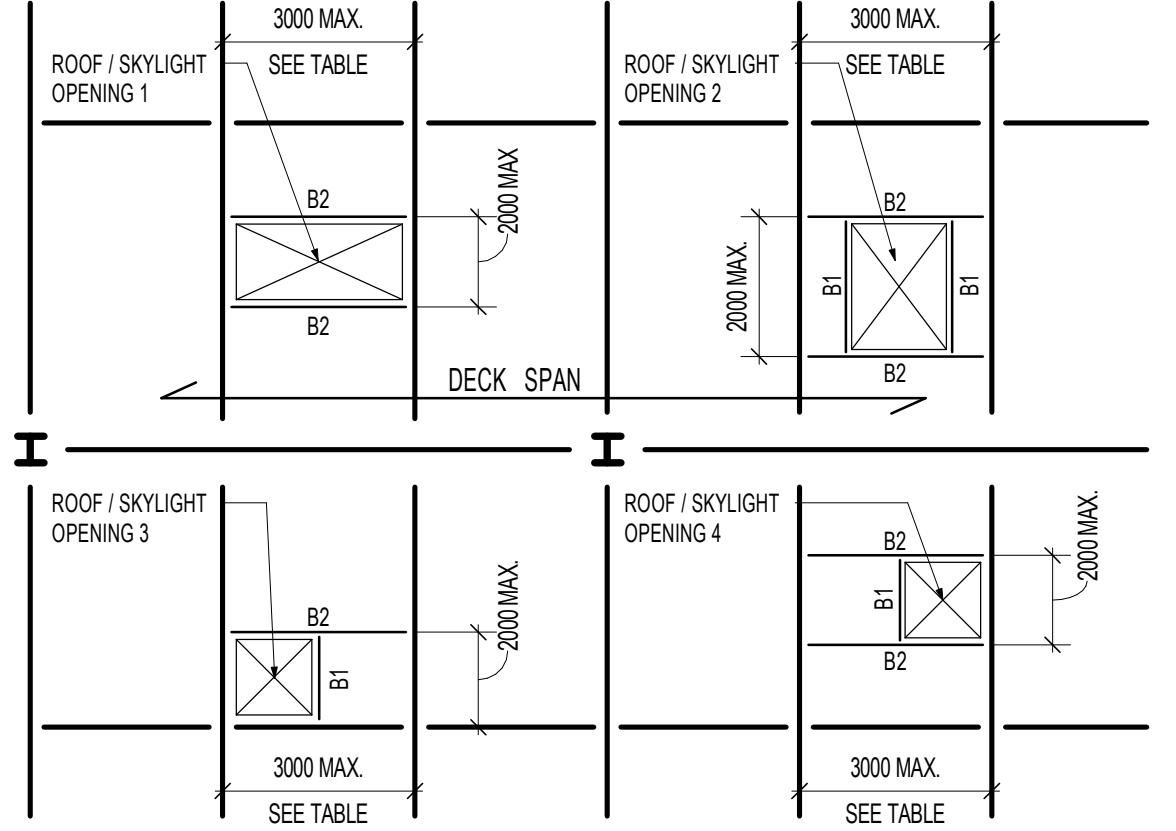
6. REFER ALSO TO GENERAL NOTES, STEEL NOTES AND CAST-IN-PLACE CONCRETE NOTES.

7. REFER TO SPLICE AND DEVELOPMENT TABLES IN C02A, C02B, C03A AND C03B.

8. REFER TO SAB02 FOR ALL ITEMS MARKED WITH *

ROOF FRAMING AT OPENINGS IN ROOF DECK

SR01



TYPICAL ROOF OPENINGS IN DECK

ROOF OPENING					
SPAN (mm)		BEAM 1 (B1)		BEAM 2 (B2)	
	SIZE	CONNECTION	SIZE	CONNECTION	
0-1500	C100x8	5 kN	C100x8	10 kN	
1500-3000	C100x8	10 kN	C150x12	20 kN	

TYPICAL SKYLIGHT OPENINGS IN DECK

SKYLIGHT OPENING 2					
SPAN (mm)		BEAM 1 (B1)		BEAM 2 (B2)	
	SIZE	CONNECTION	SIZE	CONNECTION	
0-1500	C100x8	10 kN	C150x12	15 kN	
1500-3000	C100x8	10 kN	C200x17	20 kN	

NOTES:

1. TOP OF ALL TRIMMING STEEL AT UNDERSIDE OF STEEL DECK UNLESS OTHERWISE NOTED.

2. OPENINGS FRAMES ARE DESIGNED FOR THE FOLLOWING LOADS (MAX.)
DL=0.9 kPa
SDL=0.50 kPa
SNOW=1.80 kPa

3. LOCATION OF ALL MECHANICAL UNITS AND OPENINGS THROUGH ROOF IS BASED ON INFORMATION SHOWN ON MECHANICAL DRAWINGS. THE STRUCTURAL STEEL SUB-CONTRACTOR MUST CONFIRM ALL THESE DIMENSIONS AND SIZES WITH THE MECHANICAL CONTRACTOR.

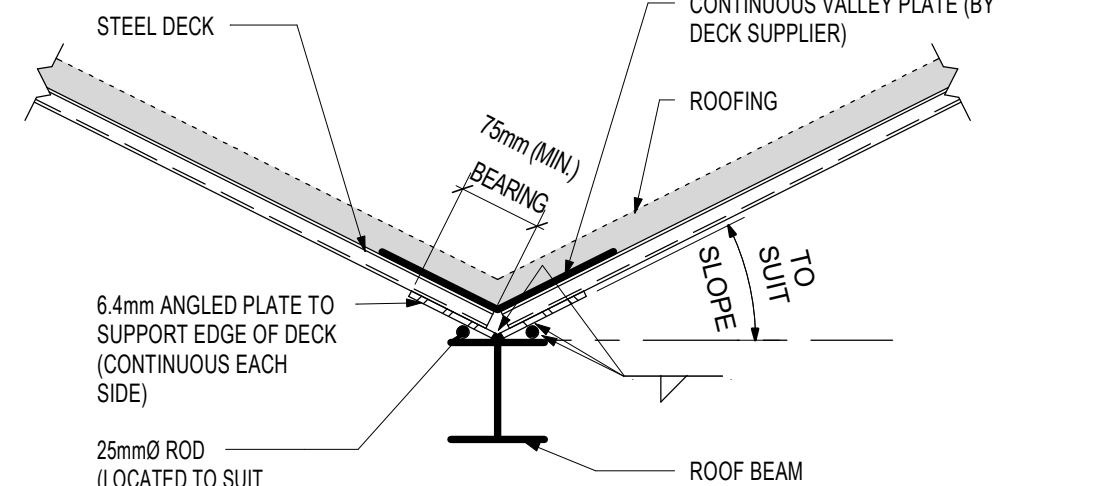
4. O.W.S.J MUST BE DESIGNED FOR ADDITIONAL LOADS FROM MECHANICAL UNITS.

5. IF ACTUAL LOCATIONS OR DETAILS VARY FROM THOSE SHOWN, THE STRUCTURAL CONSULTANT MUST BE INFORMED AND INSTRUCTIONS RECEIVED BEFORE PROCEEDING WITH THE WORK.

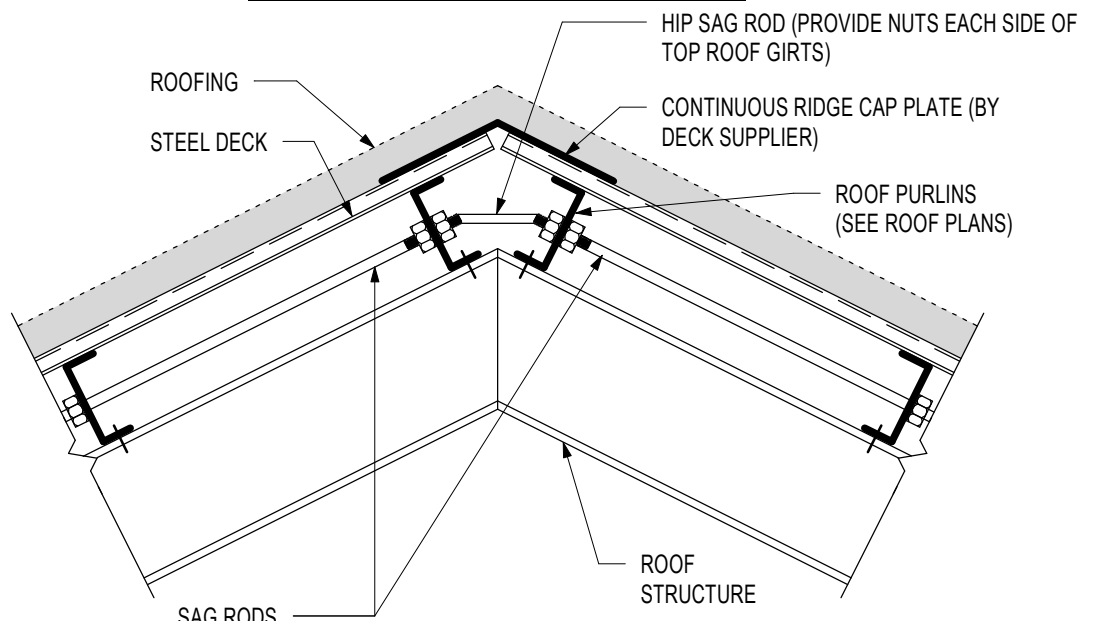
6. THE STRUCTURAL STEEL SUB-CONTRACTOR IS TO SUBMIT ERECTION DRAWINGS TO THE MECHANICAL ENGINEER AND/ OR CONTRACTOR FOR APPROVAL OF SIZE AND LOCATION OF OPENINGS FOR MECHANICAL UNITS.

VALLEY AND HIP FRAMING DETAILS

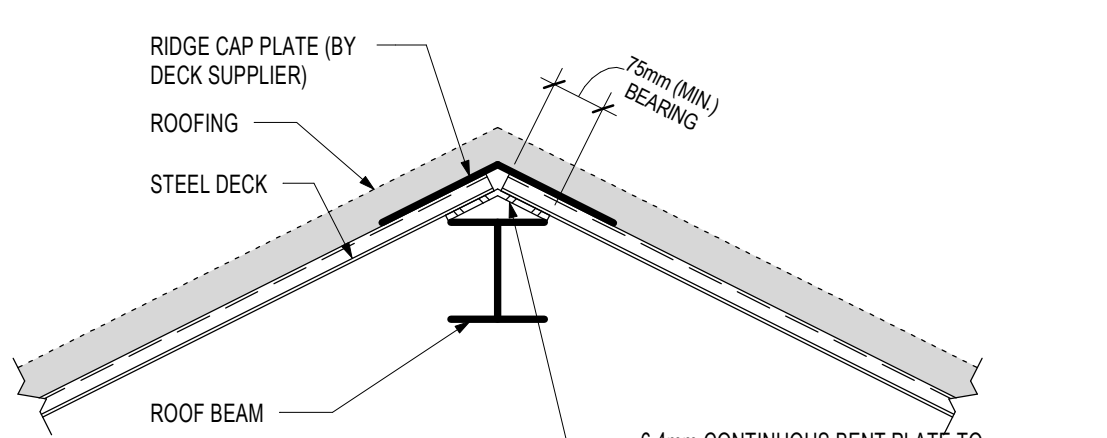
SR08



ROOF VALLEY FRAMING DETAIL



ROOF HIP FRAMING DETAIL 1



ROOF HIP FRAMING DETAIL 2

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ISSUE OR REVISION		
NO.	ISSUED FOR	DATE
1	ISSUED FOR 90% SUBMISSION	NOV/17/2020
2	ISSUED FOR ADDENDUM #1	JAN/13/2026

PROJECT: YORK REGION PRS #33
RFTC 379-21
2960 TESTON ROAD, VAUGHAN


York Region

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO COMMENCEMENT OF THE WORK. ANY DISCREPANCIES ARE TO BE REPORTED TO THE CONSULTANT.

Salas O'Brien

2235 Sheppard Ave. E. Suite No. 1100
Toronto, ON M2J 5B5

PROFESSIONAL SEAL



DWG TITLE
TYPICAL DETAILS

ORIENTATION

DATE
JAN. 2026

SCALE
1 : 1

DWG STATUS
ADDENDUM #1

PROJECT NO.
20190540

DRAWING NO.
S4-05

REVISION
2